

**EXHIBIT C-8**

**RTN 3-0021893, Malone Park Bldg No 23**

Site Information			
Site Number:	3-0021893	Category:	72 HR
Site Name:	MALONE PARK BLDG NO 23	Release Type:	RAO
Address:	200 TRAPELO RD	Current date:	8/4/2005
Town:	WALTHAM	Phase:	PHASE II
Zipcode:	02454-0000	RAO class:	
Official notification date:	6/27/2002	Location type:	STATE
Initial status date:	6/27/2003	Source:	UST
Click Here for File Viewer			

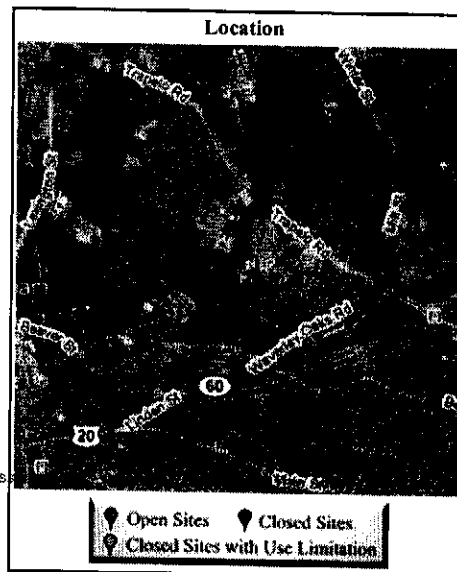
Response Action Information	
Response Action Type:	IRA - Immediate Response Action
Status:	CSRCVD - Completion Statement Received
Submittal Date:	8/4/2005
RAO class:	
Activity & Use Limitation:	
Response Action Information	
Response Action Type:	RAO - Response Action Outcome - RAO
Status:	RAORCD - RAO Statement Received
Submittal Date:	8/4/2005
RAO class:	A2
Activity & Use Limitation:	NONE
Response Action Information	
Response Action Type:	PHASEI - Phase 1
Status:	CSRCVD - Completion Statement Received
Submittal Date:	7/3/2003
RAO class:	
Activity & Use Limitation:	
Response Action Information	
Response Action Type:	TCLASS - Tier Classification
Status:	TIERII - Tier 2 Classification
Submittal Date:	7/3/2003
RAO class:	
Activity & Use Limitation:	
Response Action Information	
Response Action Type:	RNF - Release Notification Form Received
Status:	REPORT - Reportable Release or Threat of Release
Submittal Date:	8/26/2002
RAO class:	
Activity & Use Limitation:	
Response Action Information	
Response Action Type:	REL - Potential Release or Threat of Release
Status:	REPORT - Reportable Release or Threat of Release
Submittal Date:	6/27/2002
RAO class:	
Activity & Use Limitation:	

Chemicals		
Chemical	Amount	Units
FUEL OIL #2	100	PPMV
FUEL OIL #2	200	PPMV

LSPs	
LSP#	Name
8493	KLINGLER, BRIAN F

RAO Detail			
Class	Method	GW Category	Soil Category
A2	3	2	2
A2	3	2	2

Tier Classification Detail							
NRS Totals	II	III	IV	V	VI	Zone 2	Imminent Hazard
247	35	107	25	80	0	N	N
247	35	107	25	80	0	N	N





CIVIL DESIGN & LAND PLANNING  
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REGULATORY COMPLIANCE & PLANNING

February 26, 2004  
Project No. 4701.A

Mr. Roger Chu  
Bureau of Waste Site Cleanup  
Department of Environmental Protection  
Northeast Regional Office  
1 Winter Street  
Boston, Massachusetts 02108

RE: **Immediate Response Action Status Report**  
Fernald Center - Malone Park  
Building No. 23  
200 Trapelo Road  
Waltham, Massachusetts  
Release Tracking Number 3-21893

Dear Mr. Chu:

Coneco Engineers and Scientists (Coneco) has prepared the following Immediate Response Action (IRA) Status Report for the performance of IRA activities relating to a release of fuel oil from a 500-gallon underground storage tank (UST) at Building No. 23 of Malone Park, within the Fernald Center at 200 Trapelo Road in Waltham, Massachusetts, hereinafter, the "Site." The focus of the IRA is to define the limits of petroleum impacted media and reduce petroleum concentrations such that a Class A Response Action Outcome (RAO) can be achieved. This IRA Status Report has been prepared in accordance with Massachusetts Contingency Plan (MCP) specifications as set forth in 310 CMR 40.0425. This report includes a Department of Environmental Protection (DEP) IRA Transmittal Form (BWSC-105) as Appendix 1.

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## **1.0 BACKGROUND**

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On June 26, 2002, a release of fuel oil was discovered during closure activities for a 500-gallon UST formerly located at the Site. Soil samples collected from the vicinity of the UST exhibited photoionization detector (PID) headspace concentrations in excess of 100 parts per million by volume (ppmv). As a result, the DEP was verbally notified of the release on June 27, 2002, within the 72-hour notification requirement, pursuant to 310 CMR 40.0000. The release was assigned Release Tracking Number (RTN) 3-21893, and verbal approval to conduct a soil removal of up to 50 cubic yards was granted under an IRA. No holes were noted within the UST, and as such, the release was likely the result of overfilling.

## 1.1 Site Parameters

### Person Assuming

Responsibility: David Chan, Project Engineer  
Commonwealth of Massachusetts  
Department of Mental Retardation  
500 Harrison Avenue  
Boston, Massachusetts 02118  
Phone: (617) 624-7881

### Disposal Site

Limits: The release impacted soil and groundwater in the immediate vicinity of the former UST as indicated by elevated PID headspace concentrations and a petroleum sheen on groundwater within the UST excavation. As such, the Disposal Site limits include soil and groundwater located in the immediate vicinity of the former UST to a depth of approximately eleven feet below grade. Based on laboratory analytical results of soil samples collected subsequent to IRA exaction activities, impacted materials associated with this release are limited to the boundaries of the Disposal Site.

Coordinates: Latitude 42° 23' 28" N Longitude 71° 12' 59" W  
UTM 4,695,380 Meters N 317,560 Meters E (Zone 19)

### Adjacent

Properties: The Disposal Site is located within the Massachusetts Department of Mental Retardation Fernald Center, a residential and school facility. The Fernald Center is situated within a primarily residential and undeveloped area of Waltham, Massachusetts. Private residences, commercial properties, and undeveloped land surround the Site.

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## 2.0 IRA ACTIVITIES PERFORMED TO DATE

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### 2.1 IRA Excavation

On July 15 and 16, 2002, soil removal activities were conducted at the Site to remove petroleum-impacted soil and further delineate subsurface conditions. The extent to which soil was excavated was determined by the periodic screening of samples collected from the base and sidewalls using PID and standard headspace techniques in accordance with DEP Policy WSC 94-400. Overburden stratigraphy within the UST excavation consisted of fill and gravelly, silty sand. Groundwater was encountered within excavation at approximately nine feet below grade. A light sheen was noted on the surface of groundwater within the excavation. Final excavation dimensions were approximately 20 feet by 20 feet by eleven feet (length, width, depth). Impacted soil was temporarily stockpiled on-Site and covered

and lined with 6-mil polyethylene sheeting in accordance with 310 CMR 40.0030 and 310 CMR 30.0040.

Five confirmatory soil samples were collected from the excavation base and sidewalls, and submitted to Spectrum Analytical, Incorporated (Spectrum), an independent Massachusetts-certified analytical laboratory located in Agawam, Massachusetts, for analysis of extractable petroleum hydrocarbons (EPH) by the DEP Method. Original laboratory data, laboratory QA/QC, methods, and the chain-of-custody form are included as Appendix 2. A summary of the analytical data is presented in Table 1.

**Table 1 – Soil Analytical Results: EPH by the DEP Method - July 16, 2002**

Analyte	NSW (6-8')	SSW (6-8')	ESW (6-8')	WSW (6-8')	BASE (12')
C9-C18 aliphatics	ND	ND	ND	ND	400
C19-C36 aliphatics	ND	ND	63	73	250
C11-C22 aromatics	ND	ND	ND	ND	330
Fluoranthene	ND	ND	ND	0.330	ND
Pyrene	ND	ND	ND	0.330	ND

Notes: 1) ND = Not Detected above laboratory quantification limits

2) Results are reported for detected analytes only

## 2.2 Subsurface Investigation

Coneco performed a Supplemental Subsurface Investigation at the Disposal Site to confirm the presence or absence of suspected oil and/or hazardous materials within soil and groundwater at the Disposal Site.

Geoprobe® test borings were initiated at the Disposal Site on January 20, 2003, by New England Geotech of Jamestown, Rhode Island. Due to heavy snow drifting, two of the proposed test boring locations were not accessible. As such, Coneco contracted Soil Exploration of Leominster, Massachusetts to install two additional hollow stem auger borings using an all-terrain vehicle-mounted rig on June 5, 2003. As the initial monitoring well CMW-1 was destroyed during landscaping activities at the Disposal Site, a replacement well, CMW-1R, was installed on this day in the former location of boring GP-01 and well CMW-1. The locations of test borings and monitoring wells, the former UST, and other relevant Site features can be referenced in Figure 2.

Test boring locations at the Site were selected to characterize the environmental conditions within the vicinity of the former UST. Test borings were advanced to depths ranging from 11 to 13 feet below grade utilizing a truck-mounted Geoprobe® sampling system, and a hollow stem auger / split spoon sampling system. Soil samples were collected at five-foot intervals continuously in all test borings. The standard operating procedures for the performance of Geoprobe® and hollow stem auger soil borings are included as Appendix 3. The locations of test borings and monitoring wells, the former UST, and other relevant Site features can be

referenced in Figure 2. In addition, the soil-boring log is included as Appendix 4. Test boring locations are described as follows:

- GP-01: Test borings GP-01 was advanced in the location of the former 500-gallon No. 2 fuel oil UST.
- B-2: Test borings B-2 was advanced south of the former UST location. This boring was intended to describe environmental conditions of this portion of the Disposal Site and assess any potential contaminant migration outside of the former UST grave. Location selection for this boring was constrained by numerous refusals, the locations of underground electrical and drainage lines, and a tree which restricted the operation of the drilling equipment.
- B-3: Test boring B-3 was advanced west of the retaining wall located adjacent to the former UST grave in an assumed downgradient location. This boring was intended to describe environmental conditions of this portion of the Disposal Site and assess any potential contaminant migration outside of the former UST grave. Location selection for this boring was constrained by numerous refusals.

Observations made during the performance of test borings indicated the presence of fill consisting of a loamy gravelly silty sand to an average depth of two feet underlain by dense glacial/fluviol deposits of fine gravelly silty sand to a depth of approximately 13 feet below grade, the maximum depth of investigation. Groundwater and weathered bedrock were encountered at depths of six to eight and 13 feet below grade, respectively.

### 2.3 Soil Boring Screening

Representative soil samples collected from test borings B-2 and B-3 were placed in clean, tightly sealed glass jars with aluminum foil cover liners for in-field screening using a RAE Systems MiniRAE 2000 PID, calibrated to an isobutylene standard. Headspace procedures were performed in accordance with DEP Policy WSC 94-400. A discussion of this procedure and standard operating protocol is included in Appendix 3. Headspace measurements of volatile compounds (VCs) indicated concentrations of 152 parts per million (ppm) and 1.1 ppm at depths of 10 to 12 feet in borings B-2 and B-3, respectively. No additional concentrations of VCs were detected in any of the remaining soil samples.

### 2.4 Soil Boring Analytical Results

Select duplicate soil samples collected during soil boring activities were placed in the appropriate containers for analysis of EPH by the DEP Method. Sampling depth was selected to correspond to the lower extent of the vadose zone. As such, samples were collected from approximately 10 to 12 feet below grade from borings B-2 and B-3 and were labeled accordingly. All samples were shipped by courier to Spectrum on June 9, 2003. Original laboratory data, laboratory QA/QC, methods and chain of custody are provided for reference as Appendix 2. A summary of the analytical data is presented in Table 2.

**Table 2 - Soil Boring Analytical Results: EPH by the DEP Method - June 5, 2003**

Analyte	B-2 (10-12')	B-3 (10-12')	Method 1 S-2 GW-2/3 Risk Characterization Standards	Method 1 S-1 GW-2/3 Risk Characterization Standards
C9-C18 Aliphatic	450	ND	2,500	1,000
C19-C36 Aliphatic	190	ND	5,000	2,500
C11-C22 Aromatics	260	ND	2,000	800
2-Methylnaphthalene	0.780	ND	1,000	500
Fluorene	0.520	ND	2,000	1,000
Phenanthrene	1.200	ND	2,500 / 100	1,000 / 100
Fluoranthene	0.470	ND	2,000 / 1,000	1,000
Pyrene	0.660	ND	2,000	2
Notes:	Results are reported for detected analytes only in mg/Kg B-2 is subject to both S-2 GW-2 and GW-3 Risk Characterization standards B-3 is subject to solely the S-2 GW-3 Risk Characterization standards Method 1 S-1 GW 2/3 Standards are provided for potential future uses of the Disposal Site			

## 2.5 Groundwater Monitoring Well Installation

Groundwater monitoring wells CMW-1R, CMW-2, and CMW-3 were installed in test borings GP-01, B-2 and B-3, respectively. The monitoring wells were constructed of 2-inch ID, schedule 40, No. 10 slotted PVC well screen from the base of the well to approximately 2 feet below grade, with solid PVC riser pipe from the top of the slotted screen to grade. A discussion of the monitoring well installation procedure is included as Appendix 3.

## 2.6 Site Survey/Gauging of Groundwater Levels

A Site survey was conducted by Coneco personnel on June 9, 2003. The survey was performed to determine the elevation of on-Site monitoring wells. A reference elevation for each monitoring well was established at a specific point on the top of the PVC well casing. An arbitrary elevation of 100.00 feet, at the base of the southwest corner of Building No. 23, was chosen as a benchmark.

Depth to groundwater measurements were made at each groundwater monitoring well to the nearest 0.01 foot by Coneco personnel. The depth to groundwater was measured using a Keck Water Lever Indicator from the reference point located at the top of the PVC well casing. No separate-phase product was detected during the groundwater gauging. The tabulated data for the surveyed wells is presented in Table 3.

**Table 3 - Tabulation of Survey Data**

Monitoring Well	PVC Elevation	Depth to Water Table	Screen Interval	Groundwater Elevation
CMW-1R	99.06	5.16	3-13	93.90
CMW-2	99.25	7.69	3-13	91.56
CMW-3	97.89	7.99	3-11	89.90

Note: All measurements given in feet.

Groundwater surface elevation contours were computer-generated using Surfer version 7.0 Golden Software©, Inc. The groundwater contour data, as determined by groundwater gauging, is provided for reference in Figure 2. The groundwater contour plan, using data from the three monitoring wells, indicates a flow in a general south-southwesterly direction.

## 2.7 Post-Initial Excavation Groundwater Analytical Results

Groundwater samples were collected from CMW-1 on January 31, 2003 and from CMW-1R, CMW-2, and CMW-3 on June 9, 2003 for analysis of EPH by the DEP Method. Samples were collected using the applicable standard operating procedures included in Appendix 3. Laboratory analytical results and the applicable Method 1 Risk Characterization standards are summarized in Tables 4 and 5. Original laboratory data, laboratory QA/QC, methods, and chain-of-custody form are included for reference as Appendix 2.

**Table 4 - Building 23 Groundwater Analytical Results: January 31, 2003**

Analyte	CMW-1	Method 1 GW-2 Risk Characterization Standards	Method 1 GW-3 Risk Characterization Standards
C9-C18 Aliphatic	<b>110,000</b>	1,000	20,000
C19-C36 Aliphatic	<b>44,000</b>	Not Applicable	20,000
C11-C22 Aromatics	<b>103,000</b>	50,000	30,000
Naphthalene	69	6,000	6,000
2-Methylnaphthalene	200	10,000	3,000
Acenaphthene	62	Not Applicable	5,000
Fluorene	99	Not Applicable	3,000
Phenanthrene	160	Not Applicable	50
Anthracene	11	Not Applicable	3,000
Pyrene	39	Not Applicable	3,000

Notes: Results are reported for detected analytes only in µg/L  
CMW-1 is subject to both GW-2 and GW-3 Risk Characterization standards  
**BOLD** indicates concentration in excess of applicable Risk Characterization standards



**Table 5 - Building 23 Groundwater Analytical Results: June 9, 2003**

Analyte	CMW-1	CMW-2	CMW-3	Method 1 GW-2 Risk Characterization Standards	Method 1 GW-3 Risk Characterization Standards
C9-C18 Aliphatic	<b>34,000</b>	<b>1,700</b>	930	1,000	20,000
C19-C36 Aliphatic	<b>14,000</b>	690	640	Not Applicable	20,000
C11-C22 Aromatics	<b>24,000</b>	<b>1,700</b>	780	50,000	30,000
Fluorene	32	ND	ND	Not Applicable	3,000
Phenanthrene	<b>55</b>	ND	ND	Not Applicable	50
Anthracene	7.3	ND	ND	Not Applicable	3,000
Fluoranthene	5.1	ND	ND	Not Applicable	200
Pyrene	20	ND	ND	Not Applicable	3,000

Notes: Results are reported for detected analytes only in µg/L  
 CMW-1R and CMW-2 are subject to both GW-2 and GW-3 Risk Characterization standards  
 CMW-3 is subject to solely the GW-3 Risk Characterization standards  
**BOLD** indicates concentration in excess of applicable Risk Characterization standards

## 2.8 Additional IRA Excavation Activities

Based on the previously summarized analytical results, a condition of "No Significant Risk" is not present for groundwater at the Site. As such, it is the opinion of Coneco that a Response Action Outcome (RAO) cannot yet be achieved at the Site. As additional soil removal activities are necessary in order to reduce petroleum concentrations in groundwater at the Site, Coneco has completed a limited point source soil excavation within the area of concern in order to reduce concentrations of petroleum in groundwater.

On September 9 through 11, 2003, soil removal activities were conducted at the Site to remove petroleum-impacted soil. The extent to which soil was excavated was determined by the periodic screening of samples collected from the base and sidewalls using PID and standard headspace techniques in accordance with DEP Policy WSC 94-400. Overburden stratigraphy within the additional IRA excavation consisted of fill and gravelly, silty sand. Bedrock was encountered within excavation at approximately 12 feet below grade. At this depth, a true groundwater table was not encountered through moist capillary zone soil was observed. Final excavation dimensions were approximately 18 feet by 21 feet by 12 feet (length, width, depth). Approximately ten cubic yards of impacted soil was temporarily stockpiled on-Site and covered and lined with 6-mil polyethylene sheeting in accordance with 310 CMR 40.0030.

## 2.9 Post-Additional Excavation Groundwater Analytical Results

Groundwater samples were collected from CMW-1R, CMW-2, and CMW-3 on November 11, 2003 for analysis of EPH by the DEP Method. Samples were collected using the applicable standard operating procedures included in Appendix 3. Laboratory analytical results and the applicable Method 1 Risk Characterization standards are summarized in Table 6. Original laboratory data, laboratory QA/QC, methods, and chain-of-custody form are included for reference as Appendix 2.

**Table 6 - Groundwater Analytical Results: November 11, 2003**

Analyte	CMW-1R	CMW-2	CMW-3	Method 1 GW-2 Risk Characterization Standards	Method 1 GW-3 Risk Characterization Standards
C9-C18 Aliphatic	<b>2,450</b>	418	ND	1,000	20,000
C19-C36 Aliphatic	700	ND	ND	N/A	20,000
C11-C22 Aromatics	3,000	601	ND	50,000	30,000
2-Methylnaphthalene	8.80	ND	ND	10,000	3,000
Fluorene	3.65	ND	ND	N/A	3,000
Phenanthrene	5.46	ND	ND	N/A	50

Notes: Results are reported for detected analytes only in µg/L  
 CMW-1R and CMW-2 are subject to both GW-2 and GW-3 Risk Characterization standards  
 CMW-3 is subject to solely the GW-3 Risk Characterization standards  
**BOLD** indicates concentration in excess of applicable Risk Characterization standards

Groundwater samples were collected from CMW-1R, CMW-2, and CMW-3 on December 18, 2003 for analysis of EPH by the DEP Method. Samples were collected using the applicable standard operating procedures included in Appendix 3. Laboratory analytical results and the applicable Method 1 Risk Characterization standards are summarized in Table 7. Original laboratory data, laboratory QA/QC, methods, and chain-of-custody form are included for reference as Appendix 2.

**Table 7 Groundwater Analytical Results: December 18, 2003**

Analyte	CMW-1R	CMW-2	CMW-3	Method 1 GW-2 Risk Characterization Standards	Method 1 GW-3 Risk Characterization Standards
C9-C18 Aliphatic	239	919	ND	1,000	20,000
C19-C36 Aliphatic	211	502	ND	N/A	20,000
C11-C22 Aromatics	589	1,700	ND	50,000	30,000
2-Methylnaphthalene	ND	ND	ND	10,000	3,000
Fluorene	ND	ND	ND	N/A	3,000
Phenanthrene	ND	ND	ND	N/A	50

Notes: Results are reported for detected analytes only in µg/L  
 CMW-1R and CMW-2 are subject to both GW-2 and GW-3 Risk Characterization standards  
 CMW-3 is subject to solely the GW-3 Risk Characterization standards  
**BOLD** indicates concentration in excess of applicable Risk Characterization standards

Groundwater samples were collected from CMW-1R, CMW-2, and CMW-3 on February 10, 2004 for analysis of EPH by the DEP Method. Samples were collected using the applicable standard operating procedures included in Appendix 3. Laboratory analytical results and the applicable Method 1 Risk Characterization standards are summarized in Table 8. Original laboratory data, laboratory QA/QC, methods, and chain-of-custody form are included for reference as Appendix 2.

**Table 8 Groundwater Analytical Results: February 10, 2004**

Analyte	CMW-1R	CMW-2	CMW-3	Method 1 GW-2 Risk Characterization Standards	Method 1 GW-3 Risk Characterization Standards
C9-C18 Aliphatic	<b>1,260</b>	275	ND	1,000	20,000
C19-C36 Aliphatic	566	369	ND	N/A	20,000
C11-C22 Aromatics	526	ND	ND	50,000	30,000
2-Methylnaphthalene	ND	ND	ND	10,000	3,000
Fluorene	ND	ND	ND	N/A	3,000
Phenanthrene	ND	ND	ND	N/A	50

Notes: Results are reported for detected analytes only in µg/L  
CMW-1R and CMW-2 are subject to both GW-2 and GW-3 Risk Characterization standards  
CMW-3 is subject to solely the GW-3 Risk Characterization standards  
**BOLD** indicates concentration in excess of applicable Risk Characterization standards

Based on the most recent analytical results, a condition of "No Significant Risk" is not present for groundwater at the Site. As such, it is the opinion of Coneco that a Response Action Outcome (RAO) cannot yet be achieved at the Site.

## 2.10 Remediation Waste

Between July 15 and 17, 2002, approximately 40 cubic yards of impacted soil generated as part of the IRA associated with this release were removed from the Site to the ESMI, Inc. facility in Loudon, New Hampshire for proper disposal via thermal processing. The stockpiled soil was removed under a Bill of Lading associated with both the subject release and an additional separate release, identified by Release Tracking Number 3-21892, discovered at another location within Malone Park during a similar UST closure. A combined total of 121.40 tons of impacted soil were removed from the Disposal Site and the nearby release under the Bill of Lading, a copy of which is included for reference as Appendix 5.

On October 21, 2003, Coneco provided oversight for transportation and disposal activities relating to the approximately 10 cubic yard stockpile of petroleum-impacted soil generated during additional IRA excavation activities. On this day, a total of 20.18 tons of impacted soil were removed from the Site under Bill of Lading and transported by Grant to the Aggregate Industries facility in Stoughton, Massachusetts for proper disposal through asphalt batching. Remediation waste generated during additional IRA activities conducted at the Site was managed in accordance with 310 CMR 40.0034. Copies of the Soil Disposal Receipt, Certificate of Recycling, and completed Bill of Lading form (BWSC-012) are included in Appendix 5.

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### **3.0 IRA STATUS REPORT**

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#### **3.1 Regulatory Status**

Concurrent with the above-described soil and groundwater assessment activities, Coneco prepared a Phase I Initial Site Investigation and Tier Classification which was submitted to the DEP NERO on June 27, 2003. According to the completed NRS Scoresheet, the Disposal Site received a score of 247 points, which is below the Tier I cut-off score of 350 points. The Disposal Site meets none of the Tier I Inclusionary Criteria presented in 310 CMR 40.0520(2). Therefore, the Disposal Site has been classified as a Tier II Disposal Site. A copy of the June 27, 2003 Phase I Initial Site Investigation and Tier Classification is available for review at the DEP NERO.

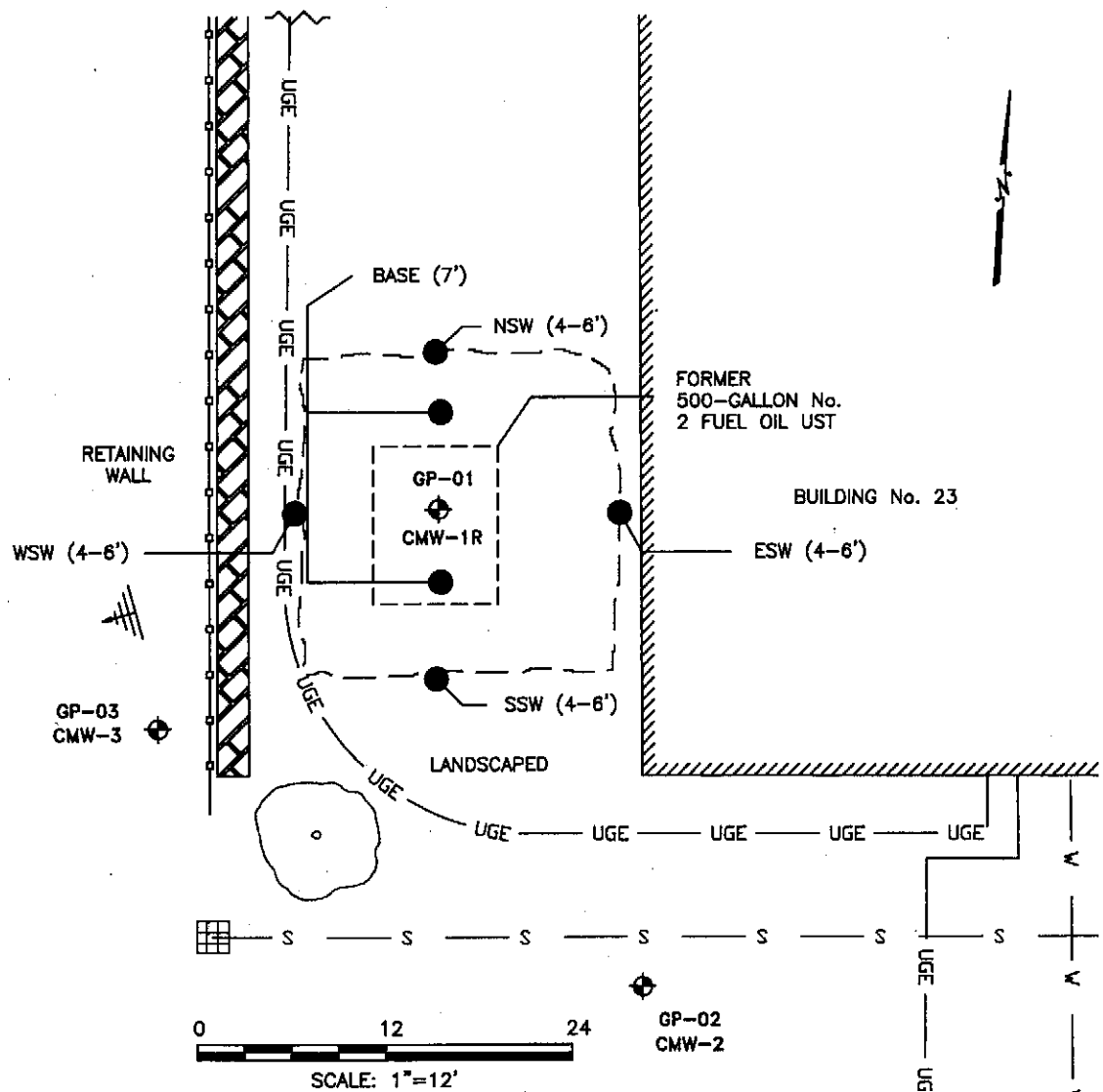
#### **3.2 Future Activities**

Based on groundwater analytical results of samples collected following the initial and additional IRA excavation activities, remedial efforts have significantly reduced concentrations of EPH in groundwater at the Site. As indicated by the most recent sampling round completed on February 10, 2004, concentrations of EPH in groundwater samples collected from the Site are in excess of the applicable GW-2/3 Method 1 Risk Characterization standards. As the DEP specifically recommends that multiple groundwater samples be taken over a period of time in order to calculate Exposure Point Concentrations for the purpose of Risk Characterization, at least four rounds of post-remediation groundwater monitoring will be completed at the Site.

Groundwater samples will be collected from monitoring wells located at the Site using appropriate DEP sampling protocols and screened in the field for temperature, pH, and specific conductivity. Samples will be submitted to an independent Massachusetts-certified laboratory for analysis of EPH using the Massachusetts DEP Method. Following the collection of this additional data, Coneco will determine if remaining petroleum concentrations at the Site, if any, represent a condition of "No Significant Risk" and meet the requirements for a Class A Response Action Outcome.

#### **3.3 Required Permits**

Pursuant to 310 CMR 40.0421, prior notice and approval from the Department is not required to conduct the above-described groundwater monitoring. An Imminent Hazard condition has not been identified at the Site. Excavated soil has been managed and disposed of in accordance with 310 CMR 40.0030 and 310 CMR 40.0034. Previous notification requirements have been completed within the timeframes required by 310 CMR 40.0032. An Immediate Response Action Transmittal Form (BWSC-105) is appended to this IRA Status Report.



### LEGEND

- |          |   |       |   |
|----------|---|-------|---|
| — — —    | APPROXIMATE EXCAVATION LIMITS               | ●     | FIELD SCREENING AND COMPOSITE CONFIRMATORY SOIL SAMPLE LOCATION |
| → UGE    | UNDERGROUND ELECTRICAL CONDUIT              | GP-01 | GEOPORBE SOIL BORING AND GROUNDWATER MONITORING WELL LOCATION   |
| — S —    | UNDERGROUND SEWER LINE                      | CMW-1 |   |
| — W —    | UNDERGROUND WATER LINE                      |       |   |
| — 90.6 — | GROUNDWATER CONTOUR WITH ELEVATIONS IN FEET |       |   |
| ← — —    | APPROXIMATE GROUNDWATER FLOW DIRECTION      |       |   |
| ☐        | STORM WATER CATCHBASIN                      | ≡     | DOWNSLOPE INDICATOR   |

FORMER 500-GALLON No. 2  
FUEL OIL UST

GP-01  
CMW-1R

93.8

RETAINING  
WALL

GP-03  
CMW-3

BUILDING  
No. 23

LANDSCAPED

92.2

91.4

90.5

GP-02  
CMW-2

0 6 12

SCALE: 1"=6'



4 FIRST STREET, BRIDGEWATER, MASSACHUSETTS (508) 697-3191

## SITE PLAN

FERNALD CENTER-MALONE PARK  
BUILDING 23  
WALTHAM, MASSACHUSETTS  
RELEASE TRACKING NUMBER 3-21893

	DRAWN	CHECKED	CAD FILE NO.	SCALE	PROJECT NO.	DRAWING NUMBER
BY	JSS	BFK	Dr./drawings/0701.31.BAP.dwg	AS NOTED	4701	FIGURE 2
DATE	01/21/03	01/21/03				